



**GUAM WATERWORKS AUTHORITY**

Gloria B. Nelson Public Service Building | 688 Route 15, Mangilao, Guam 96913

P.O. Box 3010, Hagåtña, Guam 96932

Tel. No. (671) 300-6846/48 Fax No. (671) 648-3290

Invitation For Bid: IFB-04-ENG-2026

Project: Asbestos Cement Pipe (ACP) Waterline Replacement-  
Construction, GWA Project No. 12310

Addendum No.: **04**

Date: July 9, 2026

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**All Potential Bidders:**

This addendum is issued to modify the previously issued bid documents and/or given for informational purposes and is hereby made a part of the bid documents. Failure to acknowledge receipt of this addendum shall be grounds for the bidder's disqualification and rejection of the bidder's proposal.

**1. SECTION 00100- Invitation for Bid and other sections on the bid documents where applies, which states:**

*"All sealed bids must be submitted through GWA's Portal in the format stipulated in the IFB document. Bids must be received by GWA no later than 7/17/26 3:00 PM Chamorro Standard Time. The contract time for this project is 240 calendar days for the Base Bid portion of the project. "*

*"All questions or clarifications must be submitted by email to [engbids@guamwaterworks.org](mailto:engbids@guamwaterworks.org) or by utilizing Messages in the Portal on or before 7/7/26."*

Has now been changed to read:

*"All sealed bids must be submitted through GWA's Portal in the format stipulated in the IFB document. Bids must be received by GWA no later than 7/31/26 3:00 PM Chamorro Standard Time. The contract time for this project is 240 calendar days for the Base Bid portion of the project. "*

*"All questions or clarifications must be submitted by email to [engbids@guamwaterworks.org](mailto:engbids@guamwaterworks.org) or by utilizing Messages in the Portal on or before 7/16/26."*

**2. The following sheets of Preliminary Geotechnical Assessment will now be included as supporting documentation for the project.**

Bidders are also notified to visit GWA website: <http://guamwaterworks.org/bids/> to ensure that all addenda to the bid, answers to questions, and reminders communicated are received by all bidders throughout the solicitation process.

Christopher Budasi,  
Acting General Manager

JPM

MCB; eq

Mr. Aaron Sutton  
GHD, Inc.  
316 Hernan Cortez Ave. Ste. 300  
Hagåtña, GU 96910

**PRELIMINARY GEOTECHNICAL ASSESSMENT**

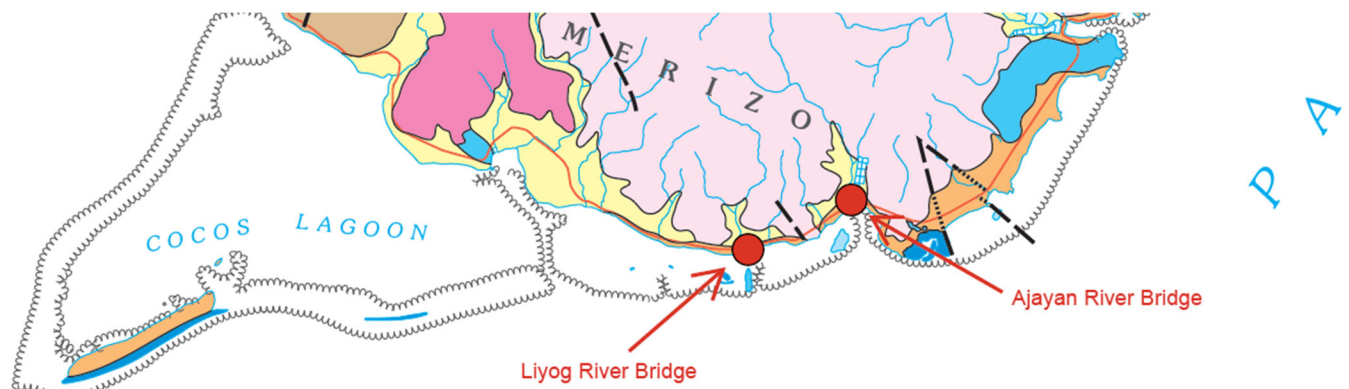
**GWA PROJECT NO. W22-05-BND : DESIGN OF ACP WATERLINE REPLACEMENT  
ROUTE 4 – AJAYAN AND LIYOG RIVER CROSSINGS, MERIZO, GUAM**

Dear Mr. Sutton,

We are pleased to provide this preliminary geotechnical assessment to support construction of horizontal directional drilling (HDD) pipeline crossings along Route 4 at the Ajayan River and Liyog River. This data report presents available geologic and geotechnical data for the two river crossings for information only during the RFP process. A geotechnical field exploration will be performed at a later date to supplement this letter report, and present site specific information.

**SITE BACKGROUND AND LOCAL GEOLOGY**

Both bridge sites are located along Route 4, at the southern end of Guam within areas mapped as Quaternary Beach Deposits (Qrb) and Quaternary Alluvium (Qal) (Figure 1). These units are mapped at or near the surface and are likely underlain by Miocene Bolanos Pyroclastic member formation (Tub).



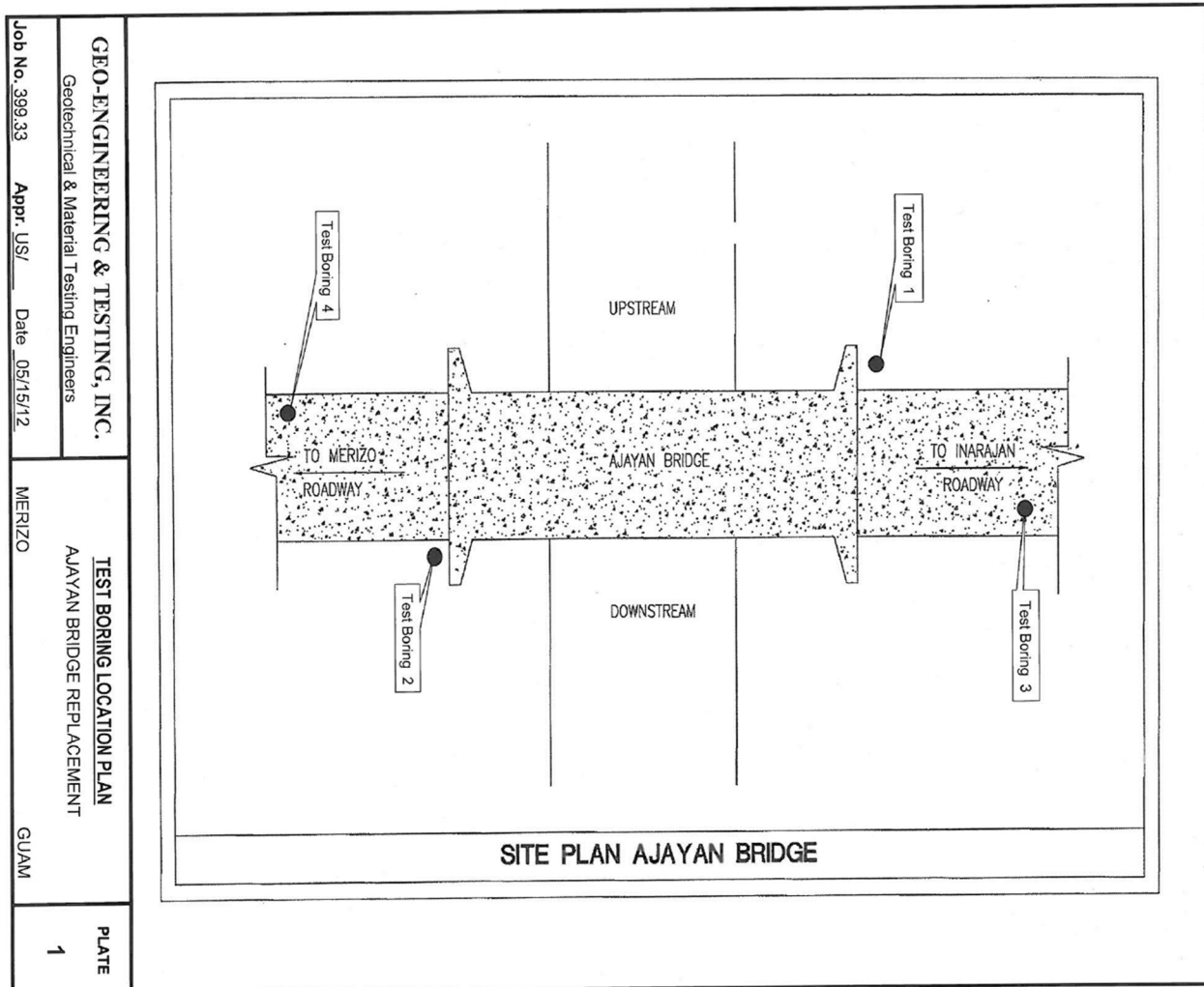
Funded by  
WATER & ENVIRONMENTAL RESEARCH INSTITUTE OF THE WESTERN PACIFIC  
through the GUAM HYDROLOGIC SURVEY PROGRAM  
**GENERALIZED GEOLOGY OF GUAM, MARIANA ISLANDS**  
H.G. Siegrist, Jr. and Mark K. Reagan  
Field interpretations assisted by Richard H. Randall and John W. Jenson  
Based on Tracey et al., 1964  
2008

- Qrb** Beach deposits (Quaternary)—beach sand and gravel, beach rock in the intertidal zone, and small isolated patches of recently emerged detrital limestone. Sand generally is less than 15 feet above sea level, seldom as much as 30 feet above
- Qal** Alluvium (Quaternary)—alluvial clay deposits, mostly 30-100 feet thick, muck and clay in marshy estuarine deposits on the west coast, scattered sand and gravel bars within deposits near SE river mouths, and clay fill in large sinks in limestone areas
- Tub** Bolanos pyroclastic member (Miocene)—breccias, conglomerates, and sandstones consisting largely of fragmented andesite. These andesites typically have prominent euhedral augite phenocrysts up to 1 centimeter in length and millimeter-scale plagioclase phenocrysts. Limestone clasts are conspicuous in some breccias and conglomerates. Estimated thickness of the Bolanos pyroclastic member ranges from 750 to 1000 feet

FIGURE 1 – GEOLOGIC SETTING

**HISTORIC GEOTECHNICAL DATA – AJAYAN RIVER CROSSING**

We reviewed a geotechnical study prepared by Geo-Engineering & Testing, Inc. (GET) in May of 2012. This study was performed for design of a replacement bridge, and included four geotechnical borings, two on either side of the bridge (*Figure 2*). Boring logs (*Attachment 1*) and laboratory test data (*Attachment 2*) are appended to this memorandum.



**FIGURE 2 – AJAYAN RIVER BORING LOCATION PLAN (GET, 2012)**

Test Borings 1 and 2, located nearest to the river, encountered fill and pavement subgrade to a depth of approximately 6 to 10 feet. Alluvial river deposits composed of loose sands and silts underlay the fill to depths between 77 and 88 feet. Tuffaceous siltstone and sandstone bedrock were observed below the Alluvial river deposits. The maximum depths of Test Borings 1 and 2 were 90 feet and 104 feet respectively.

Test Borings 3 and 4 are located an undisclosed distance from the river and were significantly shallower, having a maximum depth of 23 feet below grade. Each encountered approximately 15 feet of fill and/or pavement subgrade material over a relatively thin layer (approximately 5 feet) of alluvial deposits. Coralline gravel was encountered below the alluvial deposits.

Groundwater near the river is tidally influenced. Measurements taken during drilling showed groundwater at approximately 11 feet below grade near the river, and at approximately 8 to 10 feet below grade further from the river. Note that the ground surface elevations were not presented in the borings.

**HISTORIC GEOTECHNICAL DATA – LIYOG RIVER CROSSING**

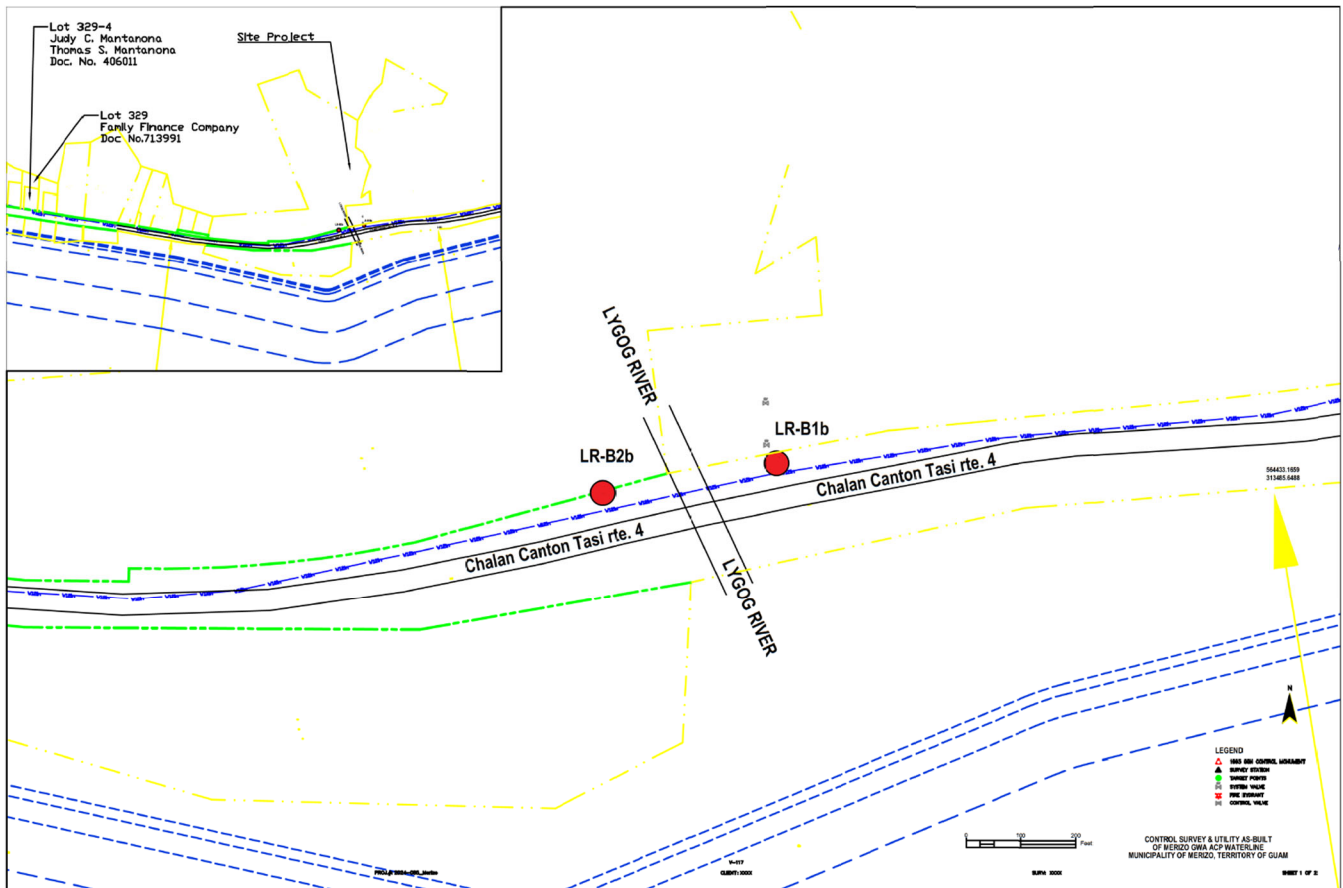
No data was provided for the Liyog River crossing, however, due to its proximity to the Ajayan River, and given they are in a similar geologic and hydrogeologic setting, conditions are likely similar to those encountered at Ajayan.

**PENDING GEOTECHNICAL EXPLORATION**

Our planned exploration will include four (4) geotechnical borings – two (2) at either side of each bridge (*Figures 3 and 4*). Borings will be drilled using rotary wash methods to approximately 50 feet below existing grade, or to refusal based on conditions observed onsite. Samples will be taken for laboratory index and strength testing at intervals of approximately five feet for the depth of drilling.

A report will be prepared that includes the following:

- Geotechnical boring logs
- Geotechnical laboratory data
- Hydrogeologic assessment



**FIGURE 3 – LIYOG RIVER EXPLORATION**

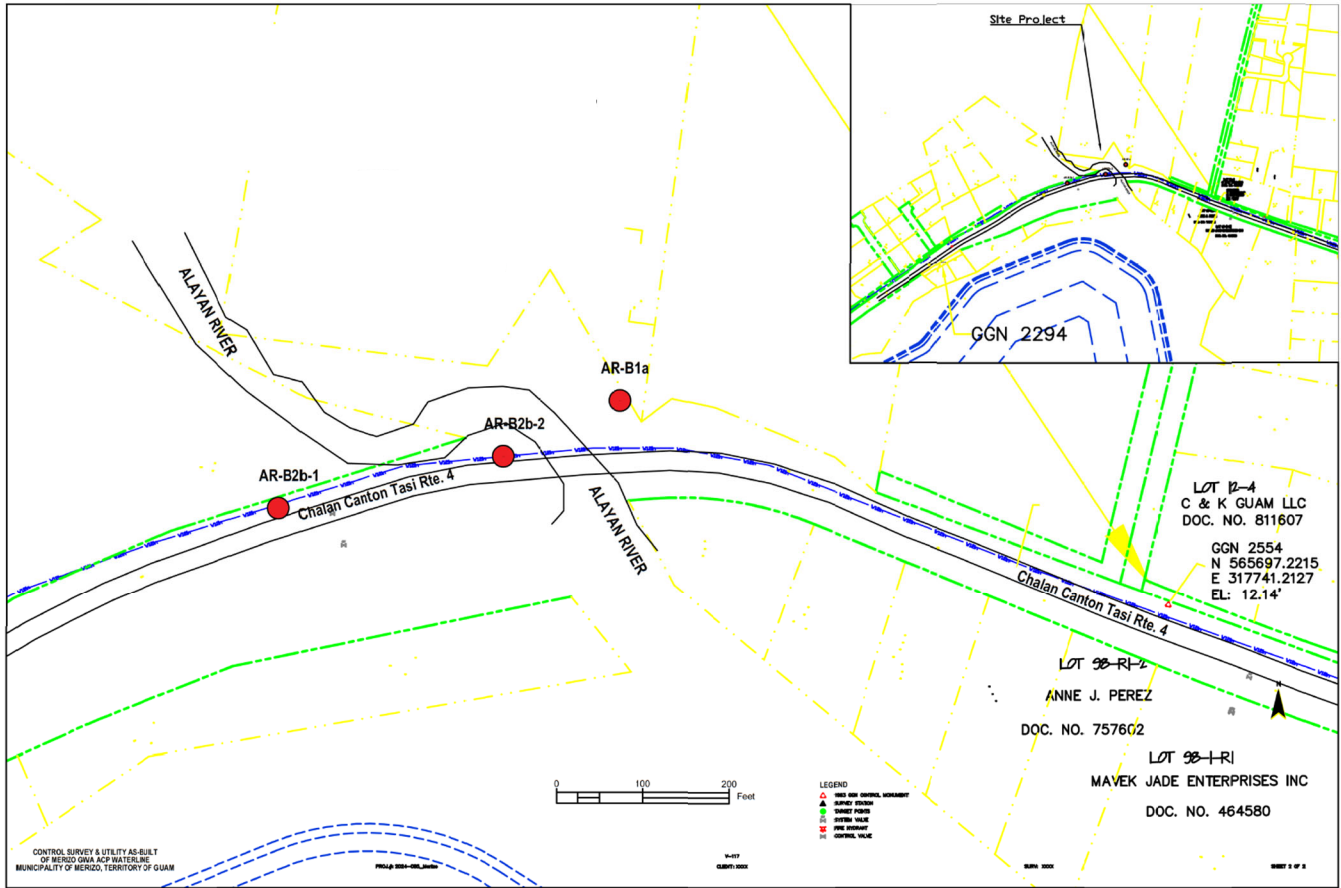


FIGURE 4 - AJAYAN RIVER EXPLORATION

**CLOSURE**

This data report is intended for use by GHD and GWA in the RFP process for Route 4 Ajayan and Liyog River Crossings project. All data presented herein is freely available to the public, or has been provided by authorized personnel for the express purpose stated above.

Agnew Consulting Engineers, L.L.C. strives to provide professional engineering services within the standard practices employed in Guam. We make no warranty, express or implied, associated with our services.

Please contact us with any questions or concerns regarding this letter report.

Sincerely,  
 Agnew Consulting Engineers, L.L.C.  
 Dustin Agnew, PE, GE

- Attachments:
- 1) Log of Test Borings
  - 2) Laboratory Data
































MAJOR DIVISIONS		SYMBOL	TYPICAL NAMES
<b>COARSE GRAINED SOILS</b> MORE THAN HALF IS LARGER THAN # 200 SIEVE	<b>GRAVELS</b>  MORE THAN HALF COARSE FRACTION IS LARGER THAN No. 4 SIEVE SIZE	CLEAN GRAVELS WITH LITTLE OR NO FINES	GW  WELL GRADED GRAVEL, WELL-GRADED GRAVEL WITH SAND
			GP  POORLY GRADED GRAVEL, POORLY GRADED GRAVEL WITH SAND
		GRAVELS WITH OVER 12 % FINES	GM  SILTY GRAVEL, SILTY GRAVEL WITH SAND
			GC  CLAYEY GRAVEL, CLAYEY GRAVEL WITH SAND
	<b>SANDS</b>  MORE THAN HALF COARSE FRACTION IS SMALLER THAN No. 4 SIEVE SIZE	CLEAN SANDS WITH LITTLE OR NO FINES	SW  WELL-GRADED SAND, WELL-GRADED SAND WITH GRAVEL
			SP  POORLY GRADED SAND, POORLY GRADED SAND WITH GRAVEL
		SANDS WITH OVER 12 % FINES	SM  SILTY SAND, SILTY SAND WITH GRAVEL
			SC  CLAYEY SAND, CLAYEY SAND WITH GRAVEL
<b>FINE GRAINED SOILS</b> MORE THAN HALF IS SMALLER THAN # 200 SIEVE	<b>SILTS AND CLAYS</b>  LIQUID LIMIT LESS THAN 50	ML  SILT, SILT WITH SAND OR GRAVEL, SANDY OR GRAVELLY SILT	
		CL  LEAN CLAY, LEAN CLAY WITH SAND OR GRAVEL, SANDY OR GRAVELLY LEAN CLAY	
		OL  ORGANIC SILT OR CLAY, ORGANIC SILT OR CLAY WITH SAND OR GRAVEL, SANDY OR GRAVELLY ORGANIC SILT OR CLAY	
	<b>SILTS AND CLAYS</b>  LIQUID LIMIT GREATER THAN 50	MH  ELASTIC SILT, ELASTIC SILT WITH SAND OR GRAVEL, SANDY OR GRAVELLY ELASTIC SILT	
		CH  FAT CLAY, FAT CLAY WITH SAND OR GRAVEL, SANDY OR GRAVELLY FAT CLAY	
		OH  ORGANIC SILT OR CLAY, ORGANIC SILT OR CLAY WITH SAND OR GRAVEL, SANDY OR GRAVELLY ORGANIC SILT OR CLAY	
<b>HIGHLY ORGANIC SOILS</b>		Pt  PEAT AND OTHER HIGHLY ORGANIC SOILS	

**UNIFIED SOIL CLASSIFICATION SYSTEM**

- SA = Sieve Analysis (ASTM C117/C136)
- LL = Liquid Limit (ASTM D4318)
- PI = Plasticity Index (ASTM D4318)
- Consol = Consolidation (ASTM D4186)
- Compaction = Compaction (ASTM D698)
- CBR = California Bearing Ratio (ASTM D1883)
- Tx = Triaxial Shear Strength Unconsolidated/Undrained (ASTM D2850)

BLOW COUNTS REPORTED FOR UNDISTURBED SAMPLES ARE CORRECTED TO REPRESENT EQUIVALENT STANDARD PENETRATION TEST BLOWS PER 305 MILLIMETERS (FOOT)

DISTURBED SAMPLE       RELATIVELY UNDISTURBED SAMPLE

<b>GEO-ENGINEERING &amp; TESTING, INC.</b> Geotechnical & Material Testing Engineers	<b>SOIL CLASSIFICATION CHART AND KEY TO TEST DATA</b>	<b>PLATE</b>
	AJAYAN BRIDGE REPLACEMENT MERIZO	GUAM <b>6</b>
Job No. <u>399.33</u> Appr. <u>U.S.</u> Date: <u>05/15/12</u>		





# Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0	0	0	0	4	18	78	
□	0	0	1	0	2	5	92	
△	0	12	12	7	17	18	34	
◇	0	20	22	6	11	14	27	

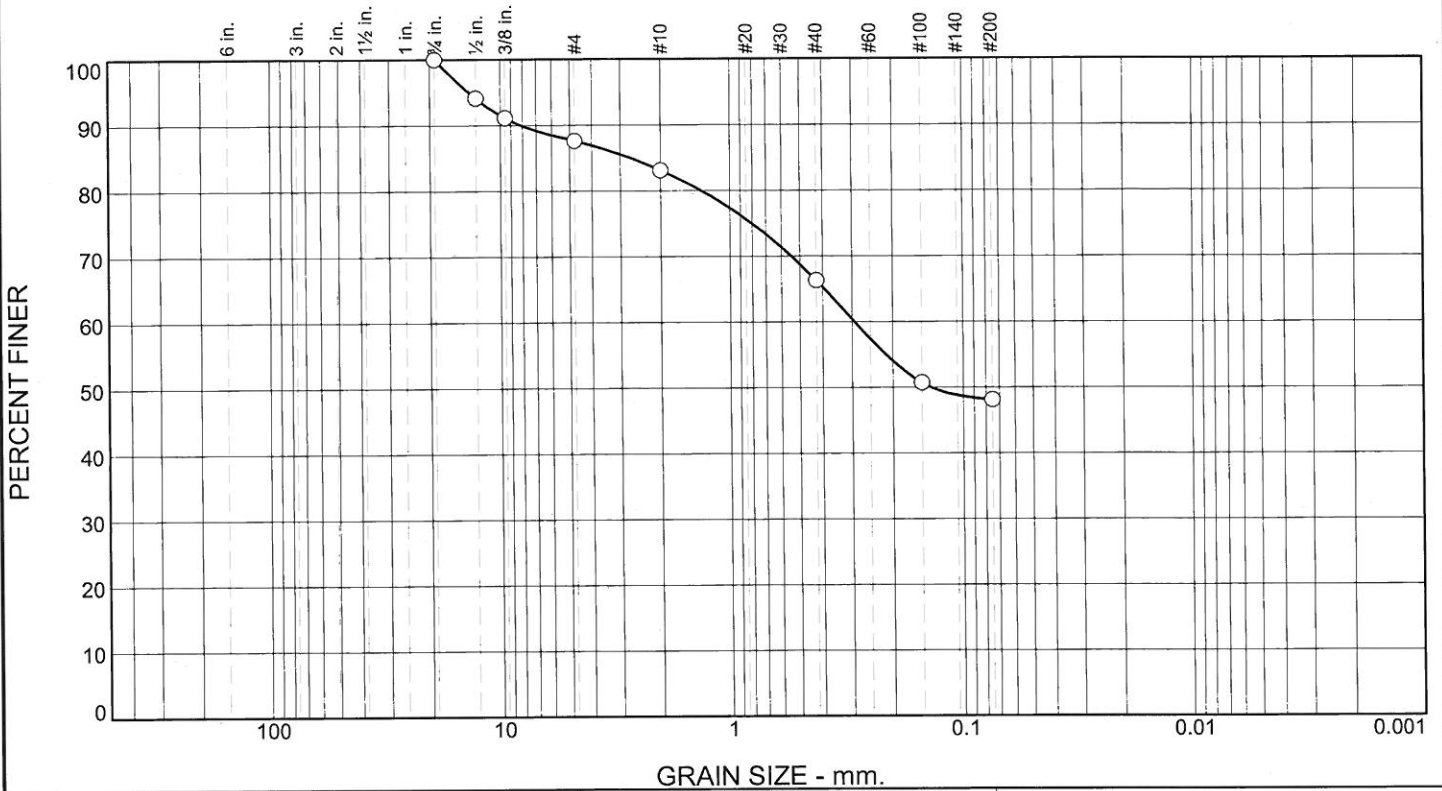
	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○			0.1362							
□	58	45								
△	46	31	14.7829	0.8251	0.3816					
◇			26.3242	6.9953	1.5142	0.1541				

Material Description	USCS	AASHTO
○ LIGHT GREY-WHITE SANDY ELASTIC SILT	MH	
□ DARK GREYISH BLACK SANDY ELASTIC SILT	MH	
△ MOTTLED REDDISH BROWN-GREY-GREEN-WHITE SILTY SAND WITH GRAVEL	SM	
◇ DARK GREY-BLACK-WHITE SILTY LIMESTONE CORAL GRAVEL WITH SAND	GM	

<b>Project No.</b> 399.33 <b>Client:</b> N. C. Macario & Associates <b>Project:</b> AJAYAN BRIDGE REPLACEMENT ○ <b>Source of Sample:</b> 2 <b>Depth:</b> 63 □ <b>Source of Sample:</b> 2 <b>Depth:</b> 73 △ <b>Source of Sample:</b> 3 <b>Depth:</b> 8 ◇ <b>Source of Sample:</b> 3 <b>Depth:</b> 12	<b>Remarks:</b>   
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**GEO-ENGINEERING & TESTING, INC.**  
 Geotechnical & Materials Testing Engineers

# Particle Size Distribution Report



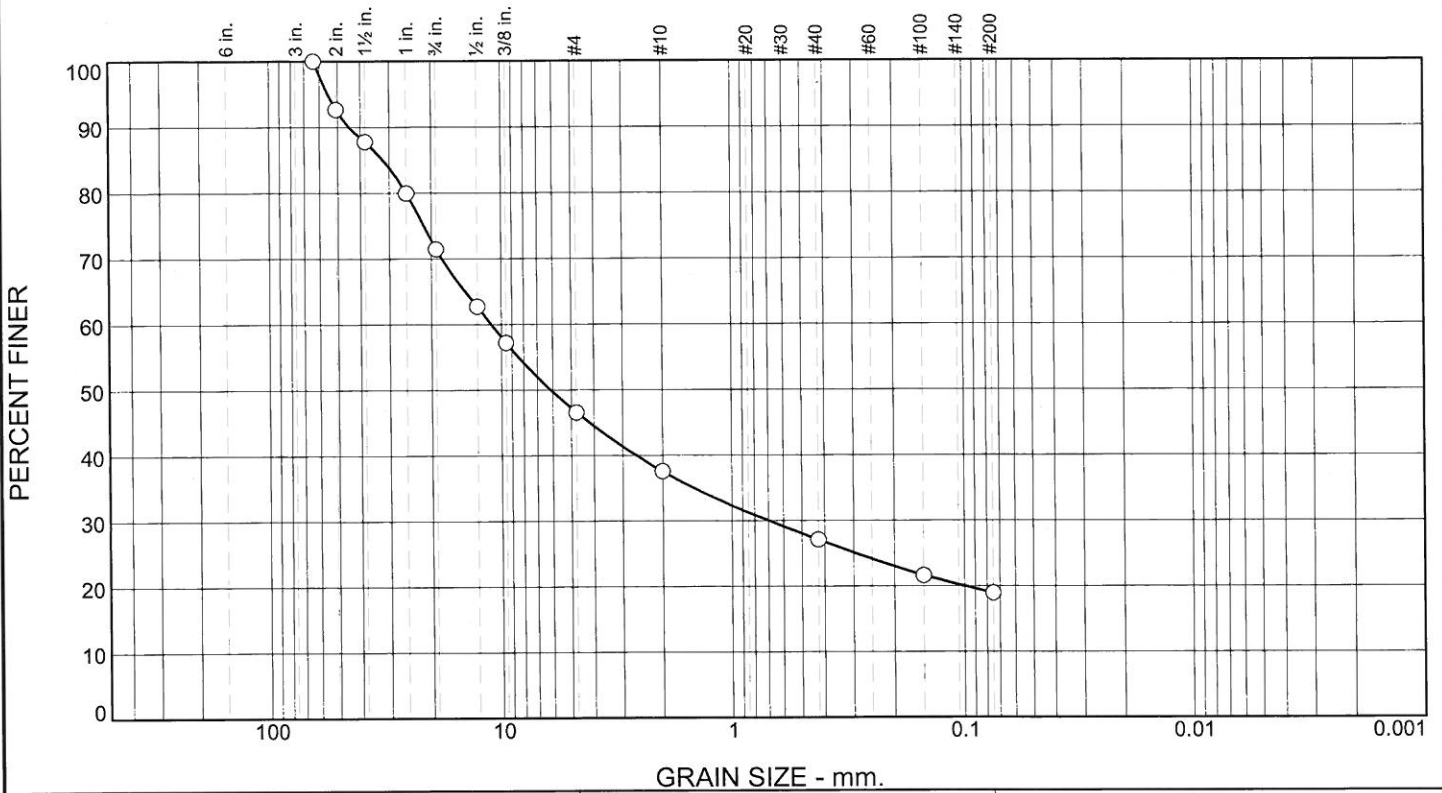
	% +3"	% Gravel		% Sand			% Fines			
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay		
<input type="radio"/>	0	0	12	5	17	18	48			
<input type="checkbox"/>	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
<input type="radio"/>	63	42	2.7256	0.2922	0.1372					

Material Description	USCS	AASHTO
<input type="radio"/> DARK GREY-RED SILTY SAND	SM	

**Project No.** 399.33      **Client:** N. C. Macario & Associates  
**Project:** AJAYAN BRIDGE REPLACEMENT  
 **Source of Sample:** 4      **Depth:** 18

**Remarks:**

# Particle Size Distribution Report



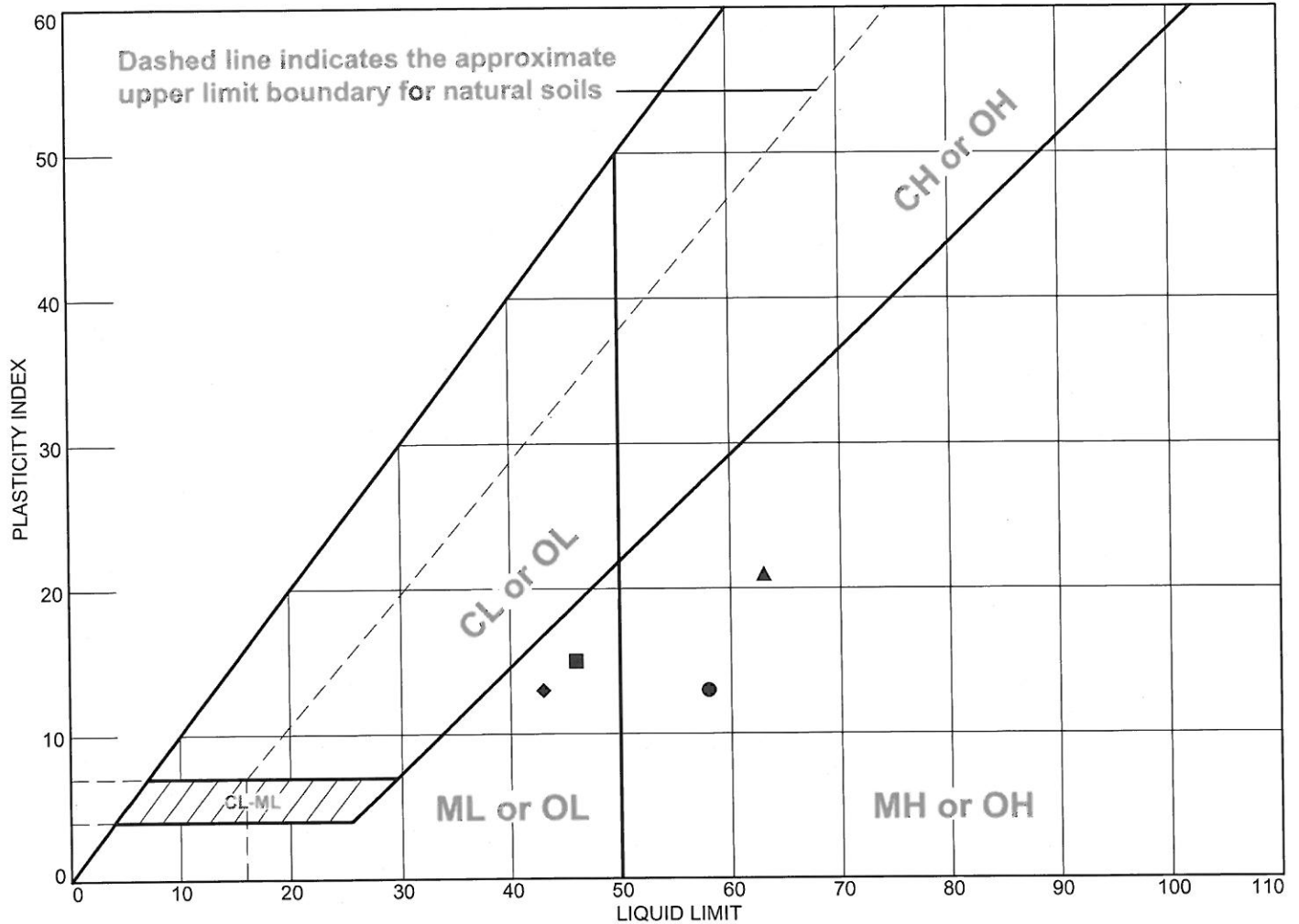
% +3"	% Gravel		% Sand			% Fines			
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay		
0	29	24	9	11	8	19			
LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
43	30	31.9336	11.0354	6.1339	0.6983				

Material Description	USCS	AASHTO
○ LIGHT BROWN-WHITE SILTY LIMESTONE GRAVEL WITH SAND	GM	

**Project No.** 399.33      **Client:** N. C. Macario & Associates  
**Project:** AJAYAN BRIDGE REPLACEMENT  
 ○ **Source of Sample:** Bulk Sample      **Depth:** 1.5

**Remarks:**

# LIQUID AND PLASTIC LIMITS TEST REPORT



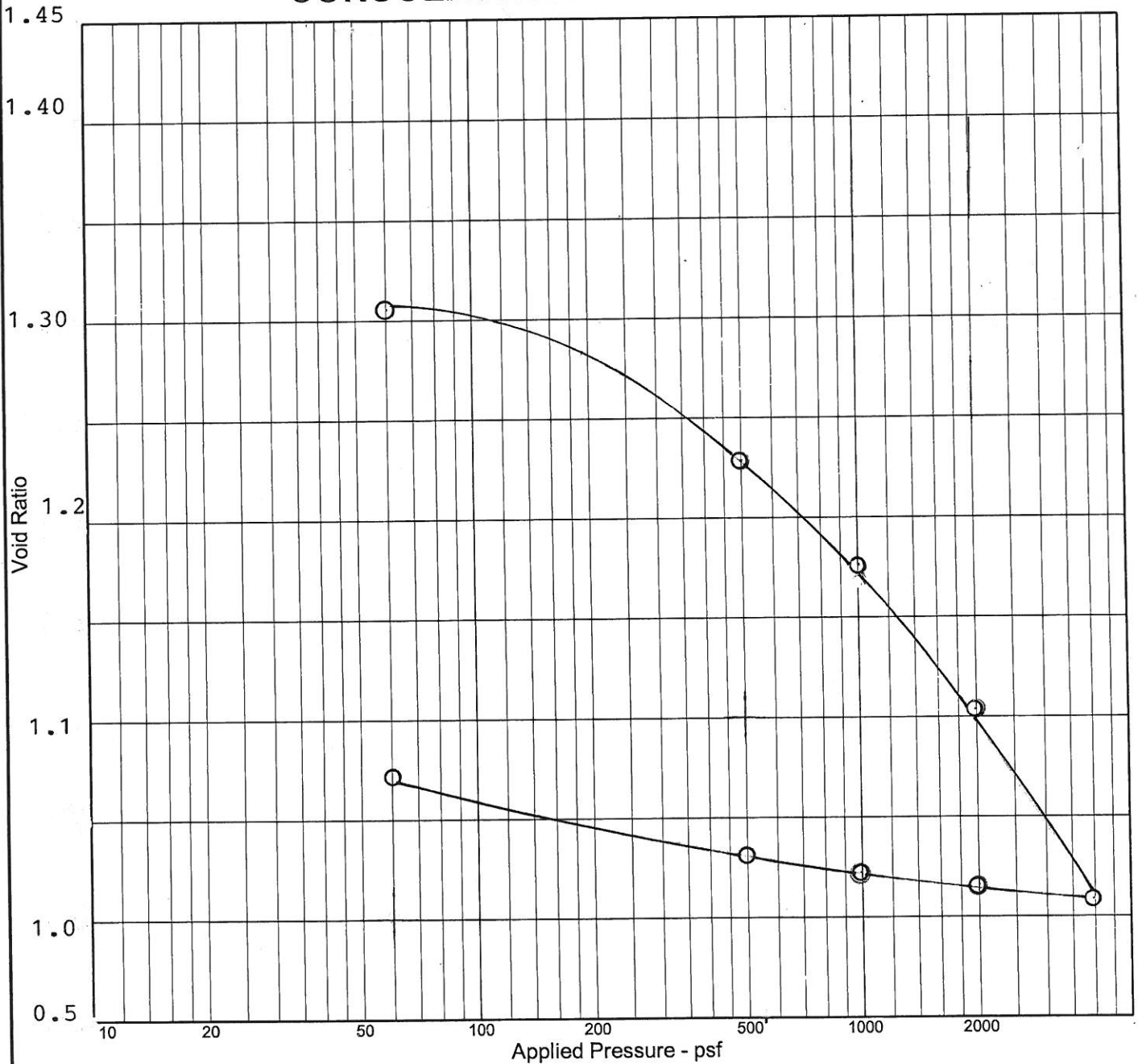
MATERIAL DESCRIPTION	LL	PL	PI	%<#40	%<#200	USCS
● DARK GREYISH BLACK SANDY ELASTIC SILT	58	45	13	97	92	MH
■ MOTTLED REDDISH BROWN-GREY-GREEN-WHITE SILTY SAND WITH GRAVEL	46	31	15	52	34	SM
▲ DARK GREY-RED SILTY SAND	63	42	21	66	48	SM
◆ LIGHT BROWN-WHITE SILTY LIMESTONE GRAVEL WITH SAND	43	30	13	27	19	GM

**Project No.** 399.33      **Client:** N. C. Macario & Associates  
**Project:** AJAYAN BRIDGE REPLACEMENT  
 MERIZO, GUAM  
 ● **Source of Sample:** 2      **Depth:** 73  
 ■ **Source of Sample:** 3      **Depth:** 8  
 ▲ **Source of Sample:** 4      **Depth:** 18  
 ◆ **Source of Sample:** Bulk Sample      **Depth:** 1.5

**Remarks:**  
 ● This test was performed on sample portion passing No. 40 sieve only.

**GEO-ENGINEERING & TESTING, INC.**  
 Geotechnical & Materials Testing Engineers

# CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (psf)	P <sub>c</sub> (psf)	C <sub>c</sub>	Initial Void Ratio
Saturation	Moisture								
103.4 %	51.9 %	70.4	63	21	2.6	1428	545	0.409	1.306

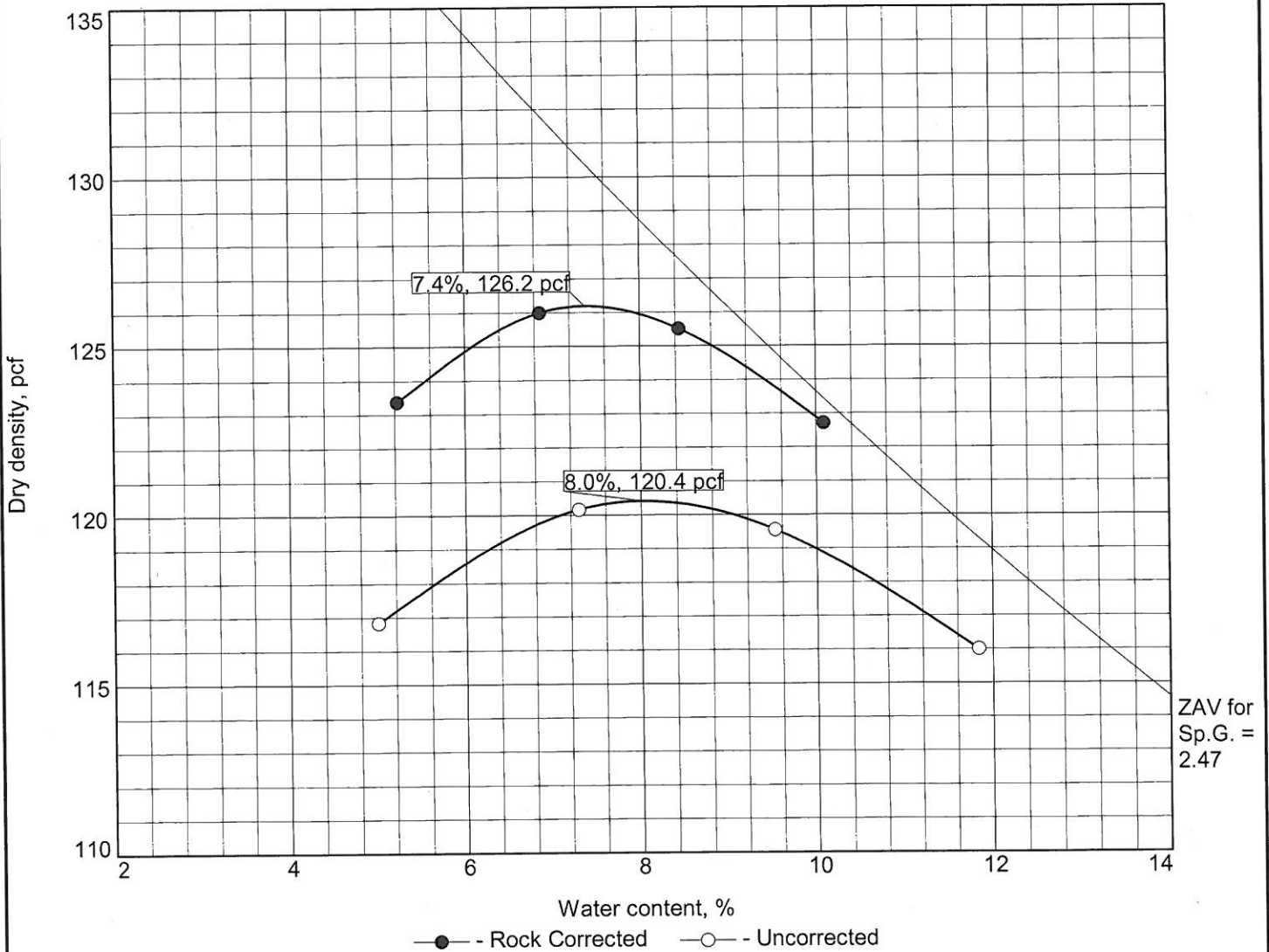
<b>MATERIAL DESCRIPTION</b>							<b>USCS</b>	<b>AASHTO</b>
dark grey-red silty sand							SM	

**Project No.** 399.33      **Client:** N. C. Macario & Associates  
**Project:** AJAYAN BRIDGE REPLACEMENT  
 MERIZO, GUAM  
**Source:** 4      **Elev./Depth:** 18  
**GEO-ENGINEERING & TESTING, INC.**  
 Geotechnical & Materials Testing Engineers

**Remarks:**  
specific gravity was assumed

Figure 13

# COMPACTION TEST REPORT For Curve No. 1



Test specification: ASTM D 1557-07 Method C Modified  
 ASTM D 4718-87 Oversize Corr. Applied to Each Test Point

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > 3/4 in.	% < No.200
	USCS	AASHTO						
1.5	GM	A-2-7(0)	11.6	2.47	43	13	29	19

ROCK CORRECTED TEST RESULTS	UNCORRECTED	MATERIAL DESCRIPTION
Maximum dry density = 126.2 pcf	120.4 pcf	LIGHT BROWN-WHITE SILTY LIMESTONE GRAVEL WITH SAND
Optimum moisture = 7.4 %	8.0 %	

**Project No.** 399.33      **Client:** N. C. Macario & Associates  
**Project:** AJAYAN BRIDGE REPLACEMENT  
 MERIZO, GUAM      **Date:** 5-22-12  
 ○ **Source of Sample:** Bulk Sample      **Depth:** 1.5

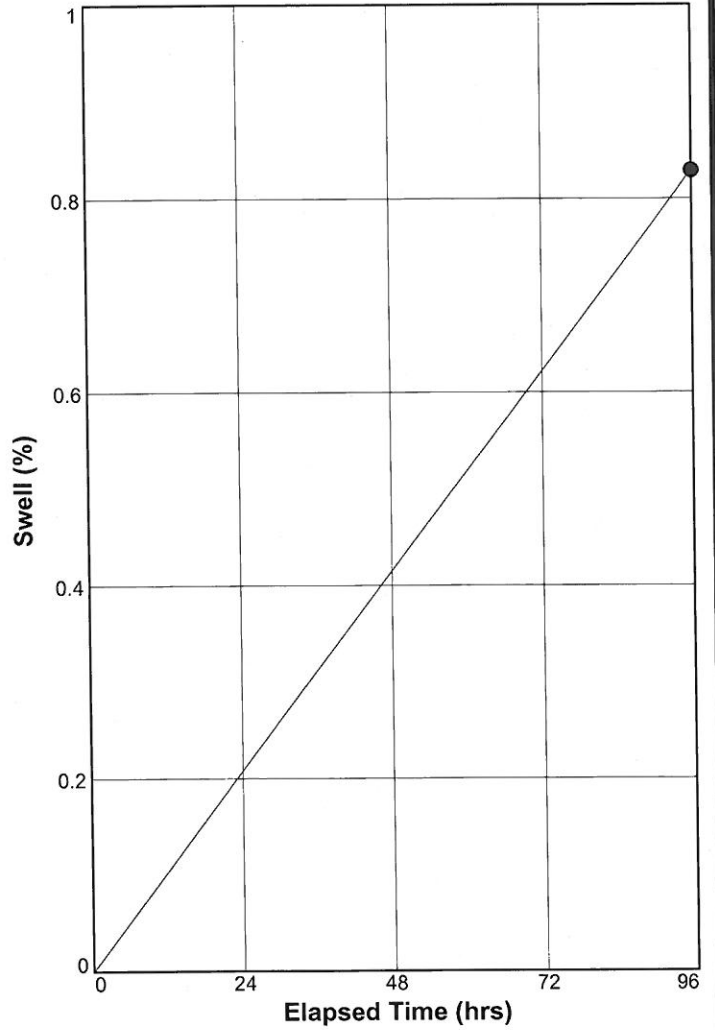
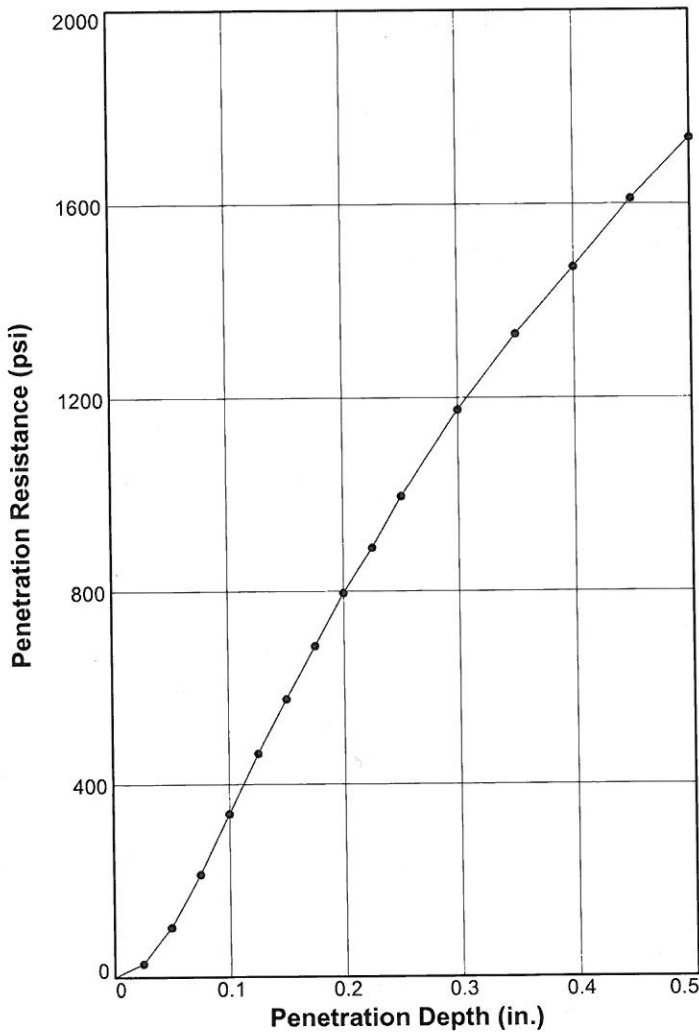
**Remarks:**

Plate 14

**GEO-ENGINEERING & TESTING, INC.**  
 Geotechnical & Materials Testing Engineers

# BEARING RATIO TEST REPORT

## ASTM D 1883-07



	Molded			Soaked			CBR (%)		Linearity Correction (in.)	Surcharge (lbs.)	Max. Swell (%)
	Density (pcf)	Percent of Max. Dens.	Moisture (%)	Density (pcf)	Percent of Max. Dens.	Moisture (%)	0.10 in.	0.20 in.			
1 ○	120.5	95.5	9.1	119.5	94.7	12.7	46.7	59.6	0.026	10	0.8
2 △											
3 □											

Material Description	USCS	Max. Dens. (pcf)	Optimum Moisture (%)	LL	PI
LIGHT BROWN-WHITE SILTY LIMESTONE GRAVEL WITH SAND	GM	126.2	7.4	43	13

**Project No:** 399.33  
**Project:** AJAYAN BRIDGE REPLACEMENT MERIZO, GUAM  
**Source of Sample:** Bulk Sample      **Depth:** 1.5  
  
**Date:**

**Test Description/Remarks:**